

### Appendix III Factor of Liquefaction Resistance, $F_L$ and Index of Liquefaction Potential, $P_L$ <sup>(7)</sup>

The ability of a soil element at an arbitrary depth to resist liquefaction can be expressed by the Factor of Liquefaction Resistance,  $F_L$ , as follows :

$$F_L = \frac{R}{L} \quad (1)$$

Where  $R$  is in-situ resistance (or undrained cyclic strength) of soil element to dynamic loads and can be simply evaluated, based on undrained cyclic shear test results, as follows :

$$R = 0.0882 \sqrt{\frac{N}{\sigma_v' + 0.7}} + 0.19, \quad \text{for } 0.02 \text{ mm} \leq D_{50} \leq 0.05 \text{ mm} \quad (2a)$$

$$R = 0.0882 \sqrt{\frac{N}{\sigma_v' + 0.7}} + 0.225 \log_{10} \frac{0.35}{D_{50}} \quad \text{for } 0.05 \text{ mm} \leq D_{50} \leq 0.6 \text{ mm} \quad (2b)$$

$$R = 0.0882 \sqrt{\frac{N}{\sigma_v' + 0.7}} - 0.05, \quad \text{for } 0.6 \text{ mm} \leq D_{50} \leq 1.5 \text{ mm} \quad (2c)$$

where  $N$  is the number of blows in the standard penetration test,  $\sigma_v'$  is the effective overburden pressure (in  $\text{kg}/\text{cm}^2$ ), and  $D_{50}$  is the mean particle diameter (in mm).  $L$  in eq. (1) is the dynamic load induced in the soil element by a seismic motion, and can be estimated by

$$L = \frac{\tau_{\max}}{\sigma_v'} = \frac{\alpha_{\max}}{g} \frac{\sigma_v}{\sigma_v'} r_d = ks \frac{\sigma_v}{\sigma_v'} r_d \quad (3)$$

Where  $\tau_{\max}$  is the maximum shear stress (in  $\text{kg}/\text{cm}^2$ ),  $\alpha_{\max}$  is the maximum acceleration on the ground

surface (in gal),  $g$  is the acceleration of gravity (= 980 gal),  $\sigma_v$  is the total overburden pressure (in  $\text{kg}/\text{cm}^2$ ),  $ks$  is the seismic coefficient, and  $r_d$ , the reduction factor for dynamic shear stress, is as follows :

$$r_d = 1 - 0.015 Z \quad (4)$$

Where  $Z$  is the depth in meters.

The Index of Liquefaction Potential  $P_L$  can be introduced to express the severity of liquefaction as,

$$P_L = \int_0^{20} F \cdot W(Z) dz \quad (5)$$

in which  $F = 1 - F_L$  for  $F_L \leq 1.0$  and  $F = 0$  for  $F_L > 1.0$ , and  $W(Z) = 10 - 0.5Z$  ( $Z$  in meter), as illustrated in Figure III-1. For the case of  $F_L = 0.0$  for the entire range from  $Z = 0$  to  $Z = 20$  m,  $P_L$  becomes 100, and for the case of  $F_L \geq 1.0$  for the entire range from  $Z = 0$  to  $Z = 20$  m,  $P_L$  becomes 0.0.

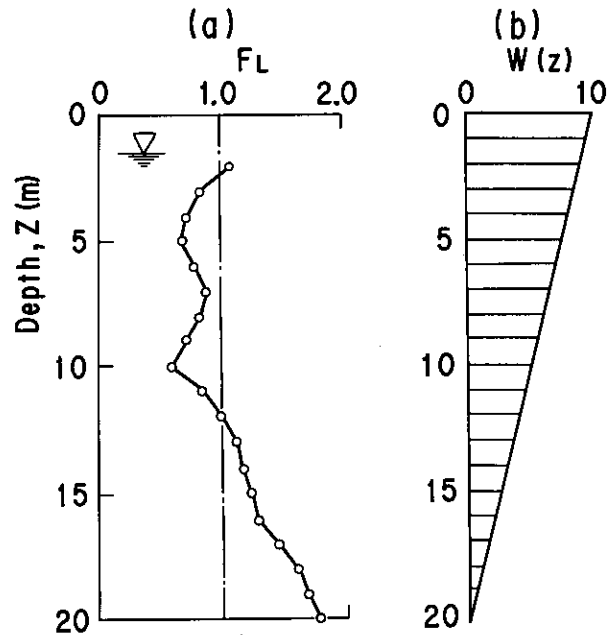


Fig. III-1 Calculation of Index of Liquefaction Potential,  $P_L$